# The Philadelphia Parking Authority 701 Market Street, Suite 5400 Philadelphia, PA 19106

# Bid No. 25-09 Surface Mount Anchors for Sign Poles Addendum Two

To: See Email Distribution List

From: Shannon Stewart

Manager of Contract Administration

Date: March 11, 2025

No Pages: 2

This addendum is issued on March 11, 2025, prior to the bid due date to add, delete, modify, clarify and/or to respond to questions submitted by Prospective Bidders regarding the work included in the above referenced solicitation.

#### SUBSTITUTION REQUEST

Substitution: Due to supply issues and potential delivery delays, we are requesting to substitute the Dewalt 3/8" x 4" Screw-Bolt+ Concrete Screw Anchors, item number: PFM1461280 with the Titen HD Heavy Duty Screw. Please see attached specifications.

Approval: Substitution is not approved.

#### **QUESTIONS**

**1. Question:** Will the Authority accept a partial bid? IE if we cannot find the match on the 10x10 base plate surface mount, will the Authority allow us to bid on the DeWalt screws?

Response: No.

**2. Question:** What type of signage is being installed on these base plates?

Response: Metal regulation signs of various sizes.

**3. Question:** Is this a previous custom fabrication?

Response: Yes.

4. Question: If these were custom fabricated, will you provide the manufacturing name?

Response: We do not have the manufacturer's name.

**5. Question:** The IFB is calling for a Dewalt item # PFM1461280 but the spec sheet in the IFB is a drawing of item # G1722408 by Zoro. Your actual picture of what you want shows Dewalt. Are these one in the same Screw-Bolt? Can we bid either the Dewalt PFM1461280 or the Zoro G1722408?

Response: Bidders must bid on Dewalt PFM1461280. G1722408 is the item number Zoro assigned to Dewalt item number PFM1461280. The drawing provided was pulled from Zoro.com for reference purposes only, bidders do not need to source Dewalt PFM1461280 from Zoro.com.

#### **END OF ADDENDUM TWO**

Cracked

Concrete

#### Titen HD® Heavy-Duty Screw Anchor



# Hex Head Mechanically Galvanized

The Titen HD heavy-duty screw anchor is a mechanically galvanized high-strength screw anchor for use in cracked and uncracked concrete, as well as uncracked masonry. Its proprietary heat treatment and ASTM B695 Class 65 mechanically galvanized coating make it ideal for both interior and exterior anchoring applications.

The Titen HD screw anchor is designed for a wide variety of applications such as sill plates, ledgers, post bases, seating, and other holdown applications. The screw anchor is easy to remove for use in temporary applications such as bracing and formwork, or when a fixture needs to be relocated.

#### **Features**

- · Thread design undercuts to efficiently transfer the load to the base material
- · Standard fractional sizes, hole size equals anchor size
- Specialized heat-treating process creates tip hardness for better cutting without compromising ductility
- · Hex washer head requires no separate washer, unless required by code
- · Fully and easily removable
- · Code listed for exterior applications

Codes: ICC-ES ESR-2713 (concrete);

ICC-ES ESR-1056 (masonry);

City of LA Supplement within ESR-2713 (concrete); City of LA Supplement within ESR-1056 (masonry);

Florida FL15730 (concrete and masonry);

FM 3017082, 3035761 and 3043442;

Multiple DOT listings

Material: Carbon steel

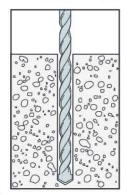
Coating: Mechanically galvanized

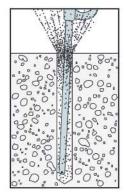
#### Additional Installation Information

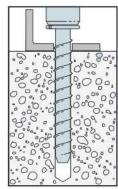
Titen HD Diameter (in.)	Wrench Size (in.)	Recommended Steel Fixture Hole Size (in.)	Minimum Hole Depth Overdrill (in.)
3/8	9/16	½ to %6	1/4
1/2	3/4	5% to 11/16	1/2
5/8	15/16	3/4 to 13/16	1/2
3/4	11/8	7/8 to 15/16	1/2

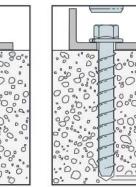
Suggested fixture hole sizes are for structural steel thicker than 12 gauge only. Larger holes are not required for wood or cold-formed steel members.

#### Installation Sequence









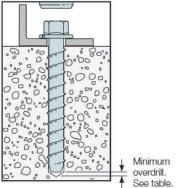


Head-stamped for easy identification



Serrated teeth on the tip of the Titen HD screw anchor facilitates cutting and reduces installation torque.

Titen HD Screw Anchor Mechanically Galvanized





Titen HD Anchor Product Data — Mechanically Galvanized

Size	Model	Thread Length	Drill Bit Diameter	Wrench Size	Qua	antity
(in.)	No.	(in.)	(in.)	(in.)	Box	Carton
% x 3	THD37300HMG	21/2			50	200
% x 4	THD37400HMG	31/2	97	0/	50	200
% x 5	THD37500HMG	41/2	- 3/8	%16	50	100
% x 6	THD37600HMG	5½			50	100
½ x 4	THD50400HMG	31/2			20	80
1/2 x 5	THD50500HMG	41/2			20	80
1/2 x 6	THD50600HMG	5½	1,1	2/	20	80
1/2 x 61/2	THD50612HMG	5½	- 1/2	3/4	20	40
1/2 x 8	THD50800HMG	5½			20	40
½ x 12	THD501200HMG	5½			5	20
% x 5	THDB62500HMG	41/2			10	40
% x 6	THDB62600HMG	5½			10	40
% x 6½	THDB62612HMG	5½	5/8	15/16	10	40
% x 8	THDB62800HMG	5½			10	20
% x 10	THDB62100HMG	5½			10	20
3/4 x 5	THD75500HMG	41/2			5	20
3/4 x 6	THDT75600HMG	41/2	97	410	5	20
3/4 x 81/2	THD75812HMG	51/2	- 3/4	11/8	5	10
3/4 x 10	THD75100HMG	5½			5	10

Mechanical galvanizing meets ASTM B695, Class 65, Type 1. Visit **strongtie.com/info** for corrosion information.

# Titen HD® Heavy-Duty Screw Anchor



#### IBC





#### Titen HD Installation Information and Additional Data<sup>1</sup>

Characteristic	Combal	Units	Nominal Anchor Diameter, d <sub>a</sub> (in.)										
Giaracteristic	Symbol	UillS	13	4	3	B		<b>½</b>	5	%	3/4		
			Installa	tion Info	rmation								
Drill Bit Diameter	d <sub>bit</sub>	in.	1	/4	3,	8	1	/2	5	<b>%</b>		3/4	
Baseplate Clearance Hole Diameter	d <sub>c</sub>	in.	3	/8	1,	2	5	⁄8	3	3/4		7/8	
Maximum Installation Torque	T <sub>inst,max</sub>	ftlbf	2	<b>4</b> <sup>2</sup>	50	)2	6	5 <sup>2</sup>	10	00²		150²	
Maximum Impact Wrench Torque Rating	T <sub>impact,max</sub>	ftlbf	12	25 <sup>3</sup>	15	O <sup>3</sup>	34	10 <sup>3</sup>	34	40³		385³	
Minimum Hole Depth	h <sub>hole</sub>	in.	13/4	25%	2¾	31/2	3¾	41/2	41/2	6	41/2	6	63/4
Nominal Embedment Depth	h <sub>nom</sub>	in.	1%	21/2	21/2	31/4	31/4	4	4	51/2	4	51/2	61/4
Critical Edge Distance	c <sub>ac</sub>	in.	3	6	211/16	3%	3%16	41/2	41/2	6%	6	6%	75/16
Minimum Edge Distance	C <sub>min</sub>	in.	1	1/2					13/4				
Minimum Spacing	Smin	in.	1	1/2	3 23/4				3				
Minimum Concrete Thickness	h <sub>min</sub>	in.	31/4	31/2	4	5	5	61/4	6	81/2	6	83/4	10
			Ade	ditional l	Data								
Anchor Category	Category	-						1					
Yield Strength	f <sub>ya</sub>	psi	100	,000					97,000	9			
Tensile Strength	f <sub>uta</sub>	psi	125,000 110,000										
Minimum Tensile and Shear Stress Area	A <sub>se</sub>	in²	0.042 0.099		0.183 0.276			276		0.414			
Axial Stiffness in Service Load Range — Uncracked Concrete	$eta_{uncr}$	lb./in.	202,000			di-		672,000	)				
Axial Stiffness in Service Load Range — Cracked Concrete	$\beta_{cr}$	lb./in.	173	,000					345,000	)			

The information presented in this table is to be used in conjunction with the design criteria of ACI 318-19 Chapter 17, ACI 318-14 Chapter 17 and ACI 318-11 Appendix D.

<sup>2.</sup> T<sub>inst,max</sub> is the maximum permitted installation torque for the embedment depth range covered by this table using a torque wrench.

<sup>3.</sup> T<sub>Impact,max</sub> is the maximum permitted torque rating for impact wrenches for the embedment depth range covered by this table.

# Titen HD® Design Information — Concrete









Titen	HD	Tension	Strength	Design	Data <sup>1</sup>
111011	1 10	101101011	Calorigari	Doorgin	Duca

O						Non	inal And	hor Dia	meter, d <sub>a</sub>	(in.)			
Characteristic	Symbol	Units	1	<b>1/4</b>		<b>%</b>	1	/2	Ē	<b>%</b>		3/4	
Nominal Embedment Depth	h <sub>nom</sub>	in.	1%	21/2	21/2	31/4	31/4	4	4	51/2	4	51/2	61/4
Steel Strength in Tens	ion — AC	1 318-1	9 17.6.1	, ACI 318	3-14 17.4	4.1 or AC	1318-11	Section	n D.5.1				
Tension Resistance of Steel	N <sub>sa</sub>	lb.	5,	195	10,	890	20,	130	30,	360		45,540	
Strength Reduction Factor — Steel Failure <sup>2</sup>	$\phi_{sa}$	-						0.65	•				
Concrete Breakout Strength i	n Tension	<sup>6</sup> — АС	I 318-19	17.6.2,	ACI 318-	14 17.4.	2 or ACI	318-11	Section	D.5.2			
Effective Embedment Depth	h <sub>ef</sub>	in.	1.19	1.94	1.77	2.40	2.35	2.99	2.97	4.24	2.94	4.22	4.86
Critical Edge Distance	cac	in.	3	6	211/16	3%	3%16	41/2	41/2	6%	6	6%	75/16
Effectiveness Factor — Uncracked Concrete	Kuncr	-	30				24				27	2	24
Effectiveness Factor — Cracked Concrete	k <sub>cr</sub>	L			1			17					
Modification Factor	$\Psi_{c,N}$							1.0					
Strength Reduction Factor — Concrete Breakout Failure <sup>2</sup>	$\phi_{cb}$	-						0.65					
Pullout Strength in Ten	sion — A	CI 318-	19 17.6.	3, ACI 31	8-14 17	.4.3 or A	CI 318-1	1 Section	on D.5.3				
Pullout Resistance, Uncracked Concrete ( $f_{\text{C}}^{*} = 2,500 \text{ psi}$ )	N <sub>p,uncr</sub>	lb.	3	3	2,7004	3	3	3	3	9,8104	3	3	3
Pullout Resistance, Cracked Concrete (f'c = 2,500 psi)	N <sub>p,cr</sub>	lb.	3	1,9054	1,2354	2,7004	3	3	3,0404	5,5704	3	6,0704	7,1954
Strength Reduction Factor — Pullout Failure <sup>2</sup>	$\phi_p$	i.—						0.65					
Tension Strength for Seismic App	lications	— ACI	318-19	17.10.3,	ACI 318-	14 17.2.	3.3 or A	CI 318-1	1 Sectio	n D.3.3.3	3		
Nominal Pullout Strength for Seismic Loads (f' $_{\text{C}} = 2,\!500$ psi)	N <sub>p,eq</sub>	lb.	3	1,9054	1,2354	2,7004	3	3	3,0404	5,5704	3,8404	6,0704	7,1954
Strength Reduction Factor — Pullout Failure <sup>2</sup>	$\phi_{eq}$	i.—						0.65			-		

- The information presented in this table is to be used in conjunction with the design criteria of ACI 318-19 chapter 17, ACI 318-14 Chapter 17 or ACI 318-11 Appendix D, except as modified below.
- 2. The strength reduction factor applies when the load combinations from the IBC or ACI 318 are used and the requirements of ACI 318-19 17.5.3, ACI 318-14 17.3.3 or ACI 318-11 D.4.3, as applicable, are met. If the load combinations of ACI 318-11 Appendix C are used, the appropriate strength reduction factor must be determined in accordance with ACI 318-11 D.4.4.
- 3. Pullout strength is not reported since concrete breakout controls.
- 4. Adjust the characteristic pullout resistance for other concrete compressive strengths by multiplying the tabular value by (f<sub>C,specified</sub> / 2,500)<sup>0.5</sup>.

## **Titen HD®** Design Information — Concrete



#### Titen HD Shear Strength Design Data1



Chamataristia	Combal	Unit				Nor	ninal And	hor Diar	neter, d <sub>a</sub>	(in.)			
Characteristic	Symbol	Unat	1	4		<b>%</b>		<u>/</u> 2	1.6	<b>%</b>		3/4	
Nominal Embedment Depth	h <sub>nom</sub>	in.	1%	21/2	21/2	31/4	31/4	4	4	51/2	4	51/2	61/4
Steel Strength in	Shear (AC	318-1	9 17.7.1	, ACI 318	3-14 17.5	5.1 or ACI	318-11	Section	D.6.1)				
Shear Resistance of Steel	V <sub>sa</sub>	lb.	2,0	)20	4,4	460	7,4	155	10,	000	14,950	16,	840
Strength Reduction Factor — Steel Failure <sup>2</sup>	$\phi_{sa}$	-4				P		0.60					
Concrete Breakout Stren	gth in Sh	ear (AC	318-19	17.7.2	ACI 318-1	14 17.5.2	or ACI 3	18-11 Se	ection D.	6.2)			
Outside Diameter	da	in.	0.	25	0.3	375	0.5	500	0.0	625		0.750	
Load Bearing Length of Anchor in Shear	$\ell_e$	in.	1.19	1.94	1.77	2.40	2.35	2.99	2.97	4.24	2.94	4.22	4.86
Strength Reduction Factor — Concrete Breakout Failure <sup>2</sup>	$\phi_{cb}$	-						0.70					
Concrete Pryout Streng	th in She	ar (ACI	318-19 1	7.7.3, A	CI 318-1	4 17.5.3	or ACI 31	8-11 Se	ction D.6	.3)			
Coefficient for Pryout Strength	k <sub>cp</sub>	lb.			1.0					2	2.0		
Strength Reduction Factor — Concrete Pryout Failure <sup>2</sup>	$\phi_{cp}$	8.8	0.70						_				
Steel Strength in Shear for Seisn	nic Applic	ations	ns (ACI 318-19 17.10.3, ACI 318-14 17.2.3.3 or ACI 318-11 Section D.3.3.3)										
Shear Resistance for Seismic Loads	V <sub>eq</sub>	lb.	1,6	1,695 2,855 4,790				8,0	000		9,350		
Strength Reduction Factor — Steel Failure <sup>2</sup>	$\phi_{eq}$	=						0.60			di .		

- 1. The information presented in this table is to be used in conjunction with the design criteria of ACI 318-19 Chapter 17, ACI 318-14 Chapter 17 and ACI 318-11 Appendix D, except as modified below.
- 2. The strength reduction factor applies when the load combinations from the IBC or ACI 318 are used and the requirements of ACI 318-19 17.5.3, ACI 318-14 17.3.3 or ACI 318-11 D.4.3, as applicable, are met. If the load combinations of ACI 318-11 Appendix C are used, the appropriate strength reduction factor must be determined in accordance with ACI 318-11 D.4.4.

#### Titen HD Tension and Shear Strength Design Data for the Soffit of Normal-Weight or Sand-Lightweight Concrete over Steel Deck<sup>1,6,7</sup>



						Nomina	I Anchor	Diamete	r, d <sub>a</sub> (in.)			
				Lower Flute Up						Upper	per Flute	
Characteristic	Symbol	Units	Figu	ıre 2	Figure 1			Figure 2		Figure 1		
			1	/4	3	<b>%</b>		/2	1	4	3⁄8	1/2
Nominal Embedment Depth	h <sub>nom</sub>	in.	1%	21/2	1%	21/2	2	31/2	1%	21/2	1%	2
Effective Embedment Depth	h <sub>ef</sub>	in.	1.19	1.94	1.23	1.77	1.29	2.56	1.19	1.94	1.23	1.29
Pullout Resistance, concrete on steel deck (cracked)2,3,4	N <sub>p,deck,cr</sub>	lb.	420	535	375	870	905	2,040	655	1,195	500	1,700
Pullout Resistance, concrete on steel deck (uncracked) <sup>2,3,4</sup>	N <sub>p,deck,uncr</sub>	lb.	995	1,275	825	1,905	1,295	2,910	1,555	2,850	1,095	2,430
Steel Strength in Shear, concrete on steel deck5	V <sub>sa, deck</sub>	lb.	1,335	1,745	2,240	2,395	2,435	4,430	2,010	2,420	4,180	7,145
Steel Strength in Shear, Seismic	V <sub>sa, deck,eq</sub>	lb.	870	1,135	1,434	1,533	1,565	2,846	1,305	1,575	2,676	4,591

- The information presented in this table is to be used in conjunction with the design criteria of ACI 318-19 Chapter 17, ACI 318-14 Chapter 17 and ACI 318-11 Appendix D, except as modified below.
- Concrete compressive strength shall be 3,000 psi minimum. The characteristic pullout resistance for greater compressive strengths shall be increased by multiplying the tabular value by (f'<sub>c,specified</sub>/3,000)<sup>0.5</sup>.
- 3. For anchors installed in the soffit of sand-lightweight or normal-weight concrete over steel deck floor and roof assemblies, as shown in Figure 1 and Figure 2, calculation of the concrete breakout strength may be omitted.
- 4. In accordance with ACI 318-19 Section 17.6.3.2.1, ACI 318-14 Section 17.4.3.2 or ACI 318-11 Section D.5.3.2, the nominal pullout strength in cracked concrete for anchors installed in the soffit of sand-lightweight or normal-weight concrete over steel deck floor and roof assemblies N<sub>p,deck,cr</sub> shall be substituted for N<sub>p,cr</sub>. Where analysis indicates no cracking at service loads, the normal pullout strength in uncracked concrete N<sub>p,deck,uncr</sub> shall be substituted for N<sub>p,uncr</sub>.
- 5. In accordance with ACI 318-19 Section 17.7.1.2(c), ACI 318-14 Section 17.5.1.2(c) or ACI 318-11 Section D.6.1.2(c), the shear strength for anchors installed in the soffit of sand-lightweight or normal-weight concrete over steel deck floor and roof assemblies V<sub>sa,deck,eq</sub> and V<sub>sa,deck,eq</sub> shall be substituted for V<sub>sa</sub>.
- 6. Minimum edge distance to edge of panel is 2her
- 7. The minimum anchor spacing along the flute must be the greater of  $3h_{\rm eff}$  or 1.5 times the flute width.

# **Titen HD®** Design Information — Concrete



Titen HD Anchor Tension and Shear Strength Design Data in the Topside of Normal-Weight Concrete or Sand-Lightweight Concrete over Steel Deck<sup>1,2,3,4</sup>

IBC		

			Nominal Anchor Diameter, d <sub>a</sub> (in.)							
Design Information	Symbol	Units	Figure 3							
			1/4	¾	18	/2				
Nominal Embedment Depth	h <sub>nom</sub>	in.	1%	21/2	31/4	4				
Effective Embedment Depth	h <sub>ef</sub>	in.	1.19	1.77	2.35	2.99				
Minimum Concrete Thickness <sup>5</sup>	h <sub>min,deck</sub>	in.	21/2	31/4	41/2	41/2				
Critical Edge Distance	Cac,deck,top	in.	3¾	71/4	9	9				
Minimum Edge Distance	C <sub>min,deck,top</sub>	in.	31/2	3	21/2	21/2				
Minimum Spacing	Smin,deck,top	in.	3½	3	3	3				

- 1. For anchors installed in the topside of concrete-filled deck assemblies, as shown in Figure 3, the nominal concrete breakout strength of a single anchor or group of anchors in shear, V<sub>cb</sub> or V<sub>cbg</sub>, respectively, must be calculated in accordance with ACI 318-19 Section 17.7.2, ACI 318-14 Section 17.5.2 or ACI 318-11 Section D.6.2, using the actual member thickness, h<sub>min,deck</sub>, in the determination of A<sub>vc</sub>.
- 2. Design capacity shall be based on calculations according to values in the tables featured on pp. 69 and 70.
- 3. Minimum flute depth (distance from top of flute to bottom of flute) is 1 1/2" (see Figure 3).
- 4. Steel deck thickness shall be minimum 20 gauge.
- 5. Minimum concrete thickness (hmin,deck) refers to concrete thickness above upper flute (see Figure 3).

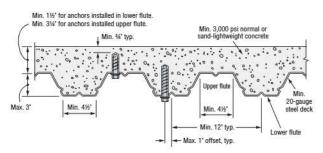


Figure 1. Installation of %"- and ½"-Diameter Anchors in the Soffit of Concrete over Steel Deck

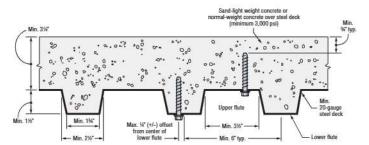


Figure 2. Installation of 1/4"-Diameter Anchors in the Soffit of Concrete over Steel Deck

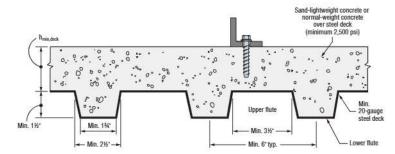


Figure 3. Installation of ¼"- and %"-Diameter Anchors in the Topside of Concrete over Steel Deck



Titen HD Allowable Tension and Shear Loads in 8" Lightweight, Medium-Weight and Normal-Weight Grout-Filled CMU

			(93)3
IBC	2472	15 EU	+

		Minimum	Critical Edge	Minimum Edge	Critical	Va	ulues for 8" Lightwe or Normal-Weight	ight, Medium-Wei Grout-Filled CMU	ght
Size in.	Drill Bit Diameter	Embedment Depth	Distance C <sub>crit</sub>	Distance C <sub>min</sub>	Spacing Distance	Tensio	n Load	Shea	r Load
(mm)	in.	in. (mm)	in. (mm)	in. (mm)	in (mm)	Ultimate Allowable lb. (kN) lb. (kN)		Ultimate lb. (kN)	Allowable lb. (kN)
	lii.		Anche	or Installed in t	he Face of the	CMU Wall (See Fig	jure 4)		
1/4 (6.4)	1/4	2½ (64)	<b>4</b> (102)	1 1/4 (32)	<b>4</b> (102)	<b>2,050</b> (9.1)	<b>410</b> (1.8)	<b>2,500</b> (11.1)	<b>500</b> (2.2)
3% (9.5)	3/8	<b>2¾</b> (70)	<b>12</b> (305)	<b>4</b> (102)	<b>6</b> (152)	<b>2,390</b> (10.6)	<b>480</b> (2.1)	<b>4,340</b> (19.3)	<b>870</b> (3.9)
½ (12.7)	1/2	3½ (89)	<b>12</b> (305)	<b>4</b> (102)	<b>8</b> (203)	<b>3,440</b> (15.3)	<b>690</b> (3.1)	<b>6,920</b> (30.8)	<b>1,385</b> (6.2)
<b>5%</b> (15.9)	5/8	<b>4½</b> (114)	<b>12</b> (305)	<b>4</b> (102)	10 (254)	<b>5,300</b> (23.6)	<b>1,060</b> (4.7)	<b>10,420</b> (46.4)	<b>2,085</b> (9.3)
3/4 (19.1)	3/4	5½ (140)	<b>12</b> (305)	<b>4</b> (102)	<b>12</b> (305)	<b>7,990</b> (35.5)	<b>1,600</b> (7.1)	<b>15,000</b> (66.7)	<b>3,000</b> (13.3)

- 1. The tabulated allowable loads are based on a safety factor of 5.0 for installations under the IBC and IRC.
- 2. Values for 8"-wide, lightweight, medium-weight and normal-weight concrete masonry units.
- 3. The masonry units must be fully grouted.

**Mechanical** Anchors

- 4. The minimum specified compressive strength of masonry, f'm, at 28 days is 1,500 psi.
- 5. Embedment depth is measured from the outside face of the concrete masonry unit.
- 6. Grout-filled CMU wall design must satisfy applicable design standards and be capable of withstanding applied loads.
- 7. Refer to allowable load-adjustment factors for spacing and edge distance on pp. 78-79.

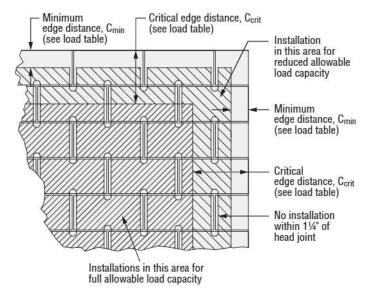


Figure 4. Shaded Area = Placement for Full and Reduced Allowable Load Capacity in Grout-Filled CMU

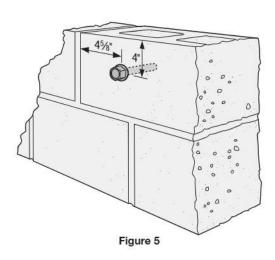


Titen HD Allowable Tension and Shear Loads in 8" Lightweight, Medium-Weight and Normal-Weight Hollow CMU

IRC	1	-	*
IDC	35 99	500 EV2	

	D. W. D.	Embedment	Minimum Edge Distance	8" Hollow CMU Loads Based on CMU Strength					
Size Drill Bit in. Diameter (mm) in.		Depth <sup>4</sup> in.		Tension Load		Shear Load			
	(mm)	in. (mm)	Ultimate lb. (kN)	Allowable lb. (kN)	Ultimate lb. (kN)	Allowable lb. (kN)			
		Anch	nor Installed in Fac	ce Shell (See Figu	re 5)				
<b>3%</b> (9.5)	3/8	<b>13/4</b> (45)	<b>4</b> (102)	<b>720</b> (3.2)	145 (0.6)	<b>1,240</b> (5.5)	250 (1.1)		
<b>½</b> (12.7)	1/2	<b>1¾</b> (45)	<b>4</b> (102)	<b>760</b> (3.4)	150 (0.7)	<b>1,240</b> (5.5)	<b>250</b> (1.1)		
<b>%</b> (15.9)	5/8	<b>1¾</b> (45)	<b>4</b> (102)	<b>800</b> (3.6)	<b>160</b> (0.7)	<b>1,240</b> (5.5)	<b>250</b> (1.1)		
<b>3/4</b> (19.1)	3/4	13/4 (45)	<b>4</b> (102)	<b>880</b> (3.9)	175 (0.8)	<b>1,240</b> (5.5)	<b>250</b> (1.1)		

- The tabulated allowable loads are based on a safety factor of 5.0 for installations under the IBC and IRC. Note: No installation within 4%" of bed joint of hollow masonry block wall.
- 2. Values for 8"-wide, lightweight, medium-weight and normal-weight concrete masonry units.
- 3. The minimum specified compressive strength of masonry, f'm, at 28 days is 1,500 psi.
- Embedment depth is measured from the outside face of the concrete masonry unit and is based on the anchor being embedded an additional ½"- through 1 ¼"-thick face shell.
- 5. Allowable loads may not be increased for short-term loading due to wind or seismic forces.
- 6. CMU wall design must satisfy applicable design standards and be capable of withstanding applied loads.
- 7. Do not use impact wrenches to install in hollow CMU.
- 8. Set drill to rotation-only mode when drilling into hollow CMU.
- 9. The tabulated allowable loads are based on one anchor installed in a single cell.
- 10. Distance from centerline of anchor to head joint shall be a minimum of 45%".



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# **Titen HD**<sup>®</sup> Design Information — Masonry



Titen HD Allowable Tension and Shear Loads in

8" Lightweight, Medium-Weight and Normal-Weight Grout-Filled CMU Stemwall



	SCANISCON.	Embed.	Minimum	Minimum	Critical	8" Gro	ut-Filled CMU A	llowable Loads I	Based on CMU St	rength, $f_m = 1$	-
Size in.	Drill Bit Diameter	Depth	Edge Distance	End Distance	Spacing Distance	Ter	sion	Shear Perpen	dicular to Edge	Shear Para	allel to Edge
(mm)	in.	in. (mm)	in. (mm)	in. (mm)	in. (mm)	Ultimate Ib. (kN)	Allowable lb. (kN)	Ultimate Ib. (kN)	Allowable lb. (kN)	Ultimate lb. (kN)	Allowable lb. (kN)
				Anchor In	stalled in C	ell Opening or \	Web (Top of Wall	) (See Figure 6)			
½ (12.7)	1/2	<b>4</b> ½ (114)	13/4 (45)	8 (203)	<b>8</b> (203)	<b>2,860</b> (12.7)	<b>570</b> (2.5)	800 (3.6)	<b>160</b> (0.7)	<b>2,920</b> (13.0)	<b>585</b> (2.6)
5/8 (15.9)	5%	<b>4½</b> (114)	13/4 (45)	10 (254)	10 (254)	<b>2,860</b> (12.7)	<b>570</b> (2.5)	800 (3.6)	160 (0.7)	<b>3,380</b> (15.0)	<b>675</b> (3.0)

- 1. The tabulated allowable loads are based on a safety factor of 5.0 for installations under the IBC and IRC.
- 2. Values are for 8"-wide, lightweight, medium-weight and normal-weight concrete masonry units.
- 3. The masonry units must be fully grouted.
- 4. The minimum specified compressive strength of masonry, f'm, at 28 days is 1,500 psi.
- 5. Grout-filled CMU wall design must satisfy applicable design standards and be capable of withstanding applied design loads.
- 6. Loads are based on anchor installed in either the web or grout-filled cell opening in the top of wall.

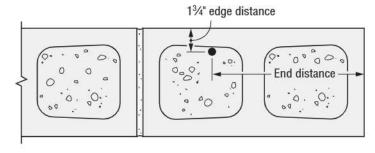


Figure 6.
Anchor Installed in Top of Wall at 134" Edge Distance

Titen HD Allowable Tension and Shear Loads in 8" Medium-Weight and Normal-Weight Grout-Filled CMU Stemwall



10.000	and I have the	Embed.	Minimum	Minimum	Critical	8" Gro	ut-Filled CMU A	llowable Loads I	Based on CMU Str	rength, f'm = 2	,000 psi
Size in.	Drill Bit Diameter	Depth	Edge Distance	End Distance	Spacing Distance	Ten	sion	Shear Perpen	dicular to Edge	Shear Para	allel to Edge
(mm)	in.	in. (mm)	in. (mm)	in. (mm)	in. (mm)	Ultimate lb. (kN)	Allowable lb. (kN)	Ultimate lb. (kN)	Allowable lb. (kN)	Ultimate lb. (kN)	Allowable lb. (kN)
				Anch	or Installed i	n Cell Opening	(Top of Wall) (S	ee Figure 7)			
½ (12.7)	1/2	41/2	3	12	12	5,800	1,160	2,750	550	7,500	1,500
5/8 (15.9)	5%	(114)	(76)	(305)	(305)	(25.8)	(5.2)	(12.2)	(2.5)	(33.4)	(6.7)

- 1. The tabulated allowable loads are based on a safety factor of 5.0 for installations under the IBC and IRC.
- 2. Values are for 8"-wide, medium-weight and normal-weight concrete masonry units.
- 3. The masonry units must be fully grouted.
- 4. The minimum specified compressive strength of masonry, f'm, at 28 days is 2,000 psi.
- 5. Allowable loads are not permitted to be increased for short-term loading due to wind or seismic forces.
- 6. Grout-filled CMU wall design must satisfy applicable design standards and be capable of withstanding applied design loads.
- 7. Loads are based on anchor installed in grout-filled cell opening in the top of wall.

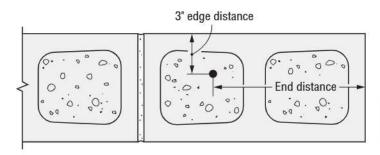


Figure 7.
Anchor Installed in Top of Wall at 3" Edge Distance

# **Titen HD**® Design Information — Masonry



Titen HD Allowable Tension and Shear Loads in End of 8" Lightweight, Medium-Weight and Normal-Weight Grout-Filled CMU Wall

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	Daill Dia	Embedment	Minimum	Minimum	Minimum	Tension Vertical Horizo		
Size (in.)	Drill Bit Diameter (in.)	Depth (in.)	Edge Distance (in.)	End Distance (in.)	Spacing (in.)	Tension (lbf)	Shear Vertical (lbf)	Shear Horizontal (lbf)
1/4	1/4	2%	313/16	13/4	4	310	215	375
3∕8	3/8	2%	313/16	1¾	6	335	215	375

- 1. The tabulated allowable loads are based on a safety factor of 5.0 for installations under the IBC and IRC.
- 2. Values are for 8"-wide, lightweight, medium-weight and normal-weight concrete masonry units.
- 3. The masonry units must be fully grouted.
- 4. The minimum specified compressive strength of masonry,  $\mathbf{f}'_{m_t}$  at 28 days is 2,000 psi.
- 5. Grout-filled CMU wall design must satisfy applicable design standards and be capable of withstanding applied design loads.
- Minimum edge and end distances are measured from anchor centerline to the edge and end of the CMU masonry wall, respectively. Refer to Figure 8 below.

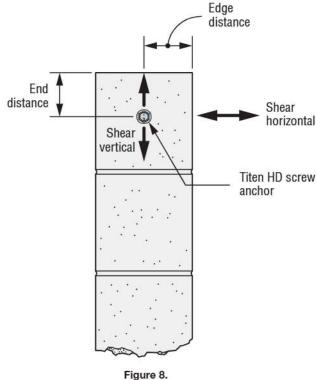


Figure 8.

Anchor Installed in
End of Grout-Filled CMU Wall

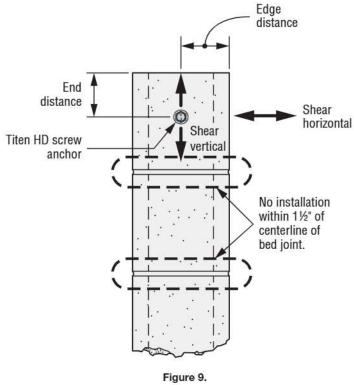


Titen HD Allowable Tension and Shear Loads in End of 8" Lightweight, Medium-Weight and Normal-Weight Hollow CMU Wall

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IBU			
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	Drill Bit Embedment	Minimum 1	Minimum Minim	Minimum	Allowable Loads			
Size (in.)	Diameter (in.)	Depth (in.)	Edge Distance (in.)	End Distance (in.)	Spacing (in.)	Tension (lbf)	Shear Vertical (lbf)	Shear Horizontal (lbf)
1/4	1/4	2%	313/16	13/4	4	130	105	120
3/8	3/8	2%	313/16	1¾	6	130	115	125

- 1. The tabulated allowable loads are based on a safety factor of 5.0 for installations under the IBC and IRC.
- 2. Values for 8"-wide, lightweight, medium-weight and normal-weight concrete masonry units.
- 3. The minimum specified compressive strength of masonry,  $f_{m_1}$  at 28 days is 2,000 psi.
- Embedment depth is measured from the outside face of the concrete masonry unit and is based on the anchor being embedded an additional 1 %"- through 1 %"-thick face shell.
- 5. Allowable loads may not be increased for short-term loading due to wind or seismic forces.
- 6. CMU wall design must satisfy applicable design standards and be capable of withstanding applied loads.
- 7. Do not use impact wrenches to install in hollow CMU.
- 8. Set drill to rotation-only mode when drilling into hollow CMU.
- Minimum edge and end distances are measured from anchor centerline to the edge and end of the CMU masonry wall, respectively. Refer to Figure 9 below.
- 10. Anchors must be installed a minimum of 1 1/2" from centerlie of bed joints, See Figure 9 for prohibited anchor installation locations,



Anchor Installed in End of Hollow CMU Wall

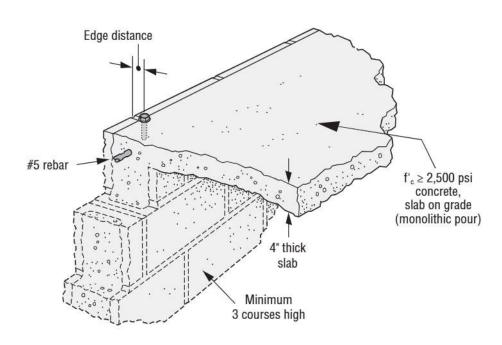


Titen HD Allowable Tension Loads for 8" Lightweight, Medium-Weight and Normal-Weight CMU Chair Blocks Filled with Normal-Weight Concrete

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Size Dril	Drill Bit	Minimum Embedment	Minimum Edge Distance in. (mm)	Critical	8" Concrete-Filled CMU Chair Block Allowable Tension Loads Based on CMU Strengtl		
in. (mm)	Diameter (in.)	Depth in. (mm)		Spacing in. (mm)	Ultimate Ib. (kN)	Allowable lb. (kN)	
		2% (60)	13/4 (44)	<b>9</b> ½ (241)	<b>3,175</b> (14.1)	<b>635</b> (2.8)	
<b>3%</b> (9.5) 3%	3% (86)	13/4 (44)	131/2 (343)	<b>5,175</b> (23.0)	<b>1,035</b> (4.6)		
		5 (127)	<b>2</b> 1/4 (57)	<b>20</b> (508)	<b>10,584</b> (47.1)	<b>2,115</b> (9.4)	
1/2	-1/	8 (203)	<b>2</b> 1/4 (57)	<b>32</b> (813)	<b>13,722</b> (61.0)	<b>2,754</b> (12.2)	
(12.7)	10 (254)	<b>2</b> 1/4 (57)	<b>40</b> (1016)	<b>16,630</b> (74.0)	<b>3,325</b> (14.8)		
<b>%</b> (15.9)	5/8	5½ (140)	13/4 (44)	<b>22</b> (559)	<b>9,025</b> (40.1)	<b>1,805</b> (8.1)	

- 1. The tabulated allowable loads are based on a safety factor of 5.0.
- Values are for 8"-wide concrete masonry units (CMÜ) filled with concrete, with minimum compressive strength of 2,500 psi and poured monolithically with the floor slab.
- 3. Center #5 rebar in CMU cell and concrete slab as shown in the illustration below.





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Load-Adjustment Factors for Titen HD Anchors in Face-of-Wall Installation in 8" Grout-Filled CMU: Edge Distance and Spacing, Tension and Shear Loads

#### How to use these charts:

- The following tables are for reduced edge distance and spacing.
- Locate the anchor size to be used for either a tension and/or shear load application.
- Locate the embedment (E) at which the anchor is to be installed.
- Locate the edge distance (c<sub>act</sub>) or spacing (s<sub>act</sub>) at which the anchor is to be installed.
- 5. The load adjustment factor (f<sub>c</sub> or f<sub>s</sub>) is the intersection of the row and column.
- Multiply the allowable load by the applicable load adjustment factor.
- Reduction factors for multiple edges or spacings are multiplied together.

Edge	Distance	Tension	$(f_{c})$

	Dia.	1/4	3/8	1/2	5/8	3/4
	E	21/2	23/4	31/2	41/2	51/2
c <sub>act</sub> (in.)	C <sub>C</sub> r	4	12	12	12	12
()	C <sub>min</sub>	1.25	4	4	4	4
	f <sub>cmin</sub>	0.77	1.00	1.00	0.83	0.66
1.25		0.77				
2		0.83				
3		0.92				
4		1.00	1.00	1.00	0.83	0.66
6		1.00	1.00	1.00	0.87	0.75
8		1.00	1.00	1.00	0.92	0.83
10		1.00	1.00	1.00	0.96	0.92
12		1.00	1.00	1.00	1.00	1.00

See footnotes below.

#### Edge Distance Shear (f<sub>c</sub>) Shear Load Parallel to Edge or End

	Dia.	1/4	3/8	1/2	5/8	3/4
ele .	E	21/2	23/4	31/2	41/2	51/2
c <sub>act</sub> (in.)	C <sub>C</sub> r	4	12	12	12	12
(111.)	c <sub>min</sub>	1.25	4	4	4	4
	f <sub>cmin</sub>	0.58	0.77	0.48	0.46	0.44
1.25		0.58				
2		0.69				
3		0.85				
4		1.00	0.77	0.48	0.46	0.44
6		1.00	0.83	0.61	0.60	0.58
8		1.00	0.89	0.74	0.73	0.72
10		1.00	0.94	0.87	0.87	0.86
12		1.00	1.00	1.00	1.00	1.00

See footnotes below.

#### Edge Distance Shear (f<sub>c</sub>) Shear Load Perpendicular to Edge or End (Directed Towards Edge or End)

c <sub>act</sub> (in.)	Dia.	1/4	3/8	1/2	5/8	3/4
	E	2½ 4 1.25 0.71	2¾ 12 4 0.58	3 ½ 12 4 0.38	4 ½ 12 4 0.30	5 ½ 12 4 0.21
	c <sub>cr</sub>					
	c <sub>min</sub> f <sub>cmin</sub>					
1.25		0.71				
2		0.79				
3		0.89				
4		1.00	0.58	0.38	0.30	0.21
6		1.00	0.69	0.54	0.48	0.41
8		1.00	0.79	0.69	0.65	0.61
10		1.00	0.90	0.85	0.83	0.80
12		1.00	1.00	1.00	1.00	1.00

- 1. E = embedment depth (inches).
- $2.c_{act}$  = actual end or edge distance at which anchor is installed (inches).
- $3. c_{cr}$  = critical end or edge distance for 100% load (inches).
- 4. c<sub>min</sub> = minimum end or edge distance for reduced load (inches).
- 5. f<sub>c</sub> = adjustment factor for allowable load at actual end or edge distance.
- 6. f<sub>ccr</sub> = adjustment factor for allowable load at critical end or edge distance. f<sub>ccr</sub> is always = 1.00.
- 7. f<sub>cmin</sub> = adjustment factor for allowable load at minimum end or edge distance.
- 8.  $f_c = f_{cmin} + [(1 f_{cmin}) (c_{act} c_{min}) / (c_{cr} c_{min})].$

# **Titen HD®** Design Information — Masonry



Load-Adjustment Factors for Titen HD Anchors in Face-of-Wall Installation in 8" Grout-Filled CMU: Edge Distance and Spacing, Tension and Shear Loads (cont.)

#### How to use these charts:

- The following tables are for reduced edge distance and spacing.
- Locate the anchor size to be used for either a tension and/or shear load application.
- Locate the embedment (E) at which the anchor is to be installed.
- Locate the edge distance (c<sub>act</sub>) or spacing (s<sub>act</sub>) at which the anchor is to be installed.
- 5. The load adjustment factor  $(f_C \text{ or } f_S)$  is the intersection of the row and column.
- Multiply the allowable load by the applicable load adjustment factor.
- Reduction factors for multiple edges or spacings are multiplied together.

Edge Distance Shear (f <sub>c</sub> )
Shear Load Perpendicular to Edge or End
(Directed Away from Edge or End)

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10000	The same a

c <sub>act</sub> (in.)	Dia. E	1.25 0.71	3% 23/4 12 4 0.89	½ 3½ 12 4 0.79	5% 4 ½ 12 4 0.58	3/4 5 1/2 12 4 0.38						
							C <sub>cr</sub>					
	c <sub>min</sub> f <sub>cmin</sub>											
							1.25		0.71			
	2							0.79				
3		0.89										
4		1.00	0.89	0.79	0.58	0.38						
6		1.00	0.92	0.84	0.69	0.54						
8		1.00	0.95	0.90	0.79	0.69						
10		1.00	0.97	0.95	0.90	0.85						
12		1.00	1.00	1.00	1.00	1.00						

#### Spacing Tension (f<sub>s</sub>)



s <sub>act</sub> (in.)	Dia. E	2½ 4 2 0.66	3/8 23/4 6 3 0.87	½ 3½ 8 4 0.69	5% 4 ½ 10 5 0.59	34 5 ½ 12 6 0.50
	s <sub>min</sub>					
2						
3		0.83	0.87			
4		1.00	0.91	0.69		
5			0.96	0.77	0.59	
6			1.00	0.85	0.67	0.50
8				1.00	0.84	0.67
10					1.00	0.83
12						1.00

#### Spacing Shear (f<sub>s</sub>)



s <sub>act</sub> (in.)	Dia.	1/4 21/2 4 2 0.87	3/8 23/4 6 3 0.62	½ 3½ 8 4 0.62	5% 4 ½ 10 5 0.62	3/4 5 ½ 12 6 0.62
	Smin					
2						
3		0.93	0.62			
4		1.00	0.75	0.62		
5			0.87	0.72	0.62	
6			1.00	0.81	0.70	0.62
8				1.00	0.85	0.75
10					1.00	0.87
12						1.00

<sup>1.</sup> E = embedment depth (inches).

<sup>2.</sup> s<sub>act</sub> = actual spacing distance at which anchors are installed (inches).

<sup>3.</sup> s<sub>cr</sub> = critical spacing distance for 100% load (inches).

<sup>4.</sup> s<sub>min</sub> = minimum spacing distance for reduced load (inches).

<sup>5,</sup> f<sub>s</sub> = adjustment factor for allowable load at actual spacing distance.

 $<sup>6.\,</sup>f_{SCT}$  = adjustment factor for allowable load at critical spacing distance.  $f_{SCT}$  is always = 1.00.

<sup>7.</sup>  $f_{\textit{smin}}$  = adjustment factor for allowable load at minimum spacing distance.

<sup>8.</sup>  $f_s = f_{smin} + [(1 - f_{smin}) (s_{act} - s_{min}) / (s_{cr} - s_{min})].$